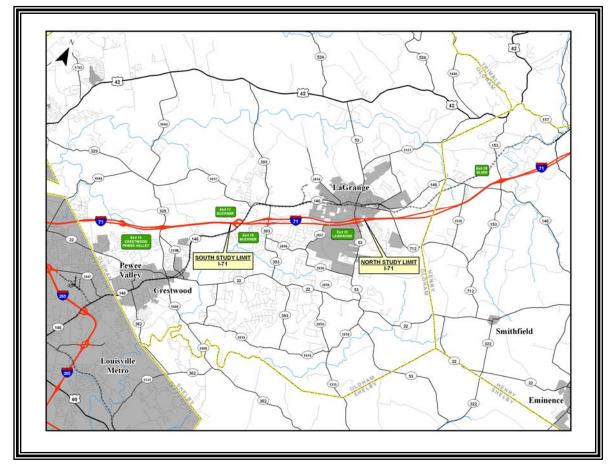
I-71 / Proposed Overpass Interchange **FEASIBILITY STUDY**

I-71 between KY 393 (Exit 18) and KY 53 (Exit 22) Oldham County, Kentucky Item No.: 05-8201.00

Final Report November 2008



Prepared for:



Kentucky Transportation Cabinet District-5, Louisville, Kentucky

Prepared by:



815 West Market Street Louisville, Kentucky

TABLE OF CONTENTS

EXE	EXECUTIVE SUMMARY1					
1.0	INTRODUCTION	3				
	1.1 Purpose of the Study	3				
	1.2 Project Background	3				
	1.3 Project Purpose and Need	4				
2.0	EXISTING CONDITIONS	6				
	2.1 Project Location	6				
	2.2 Roadway Characteristics	6				
	2.3 Traffic Conditions	6				
	2.4 Level of Service	7				
	2.5 Existing Interchange Geometry					
	2.6 Crash Analysis	8				
30	PROJECT TEAM INPUT	10				
0.0	3.1 Project Team Meetings					
	3.2 Constraint Identification					
4.0	STUDY ALTERNATIVES CONSIDERED	12				
	4.1 Transportation System Management	.12				
	4.2 Spot Improvements	.12				
	4.3 Independent System Improvements	12				
	4.4 Design Alternatives	.13				
	4.4.1 Alternatives Eliminated from Detail Study					
	4.5 Alternative Comparison	15				
5.0	CONCLUSIONS	.17				
	5.1 Recommendation	.17				
	5.2 Next Steps	.17				
	Policy Statement No. 1: Existing Facilities Capability	.17				
	Policy Statement No. 2: Transportation System Management	18				
	Policy Statement No. 3: Operational Analysis	.18				
	Policy Statement No. 4: Access Connections and Design					
	Policy Statement No. 5: Transportation and Land Use Plans	19				
	Policy Statement No. 6: Comprehensive Interstate Network Study					
	Policy Statement No. 7: Coordination with Transportation System Improvements					
	Policy Statement No. 8: Status of Planning and NEPA					
	5.2.2 NEPA Requirements					
	5.2.3 Preliminary Engineering					
	5.2.4 Public Involvement	20				

LIST OF TABLES

	Benefits Matrix	ES 2
Table 1	2008, 2015 and 2030 Traffic Conditions	8
Table 2	Existing Interchange Geometry	8
Table 3	Crash Analysis Summary	9
Table 4	Benefits Matrix	16

• APPENDICES

- Appendix A Crash Analysis
- Appendix B Traffic Volumes and LOS Summary
- Appendix C No Build
- Appendix D Alternative 1 Standard Diamond
- Appendix E Alternative 2 Collector-Distributor
- Appendix F Alternative 3 Improve Existing Roads
- Appendix G Alternative 4 New Road South and Parallel to I-71

EXECUTIVE SUMMARY

The I-71 Interchange Feasibility Study in Oldham County was prepared to assist the Kentucky Transportation Cabinet (KYTC) in evaluating the feasibility of constructing an interchange on I-71 at a proposed overpass (MP 20.6) between KY 393 (Exit 18) and KY 53 (Exit 22). A project study team approach was used, consisting of representatives from the KYTC Central Office, KYTC District 5, the

Kentuckiana Regional Planning and Development Agency (KIPDA), Oldham County Planning, Oldham County Economic Development Authority (OCEDA), and Qk4.

The project area is the I-71 corridor between KY 146 (Exit 17) and KY 53 (Exit 22) in Oldham County, Kentucky.

No improvements are included in KIPDA's Transportation Improvement Program (TIP) or Horizon 2030 Long-Range Transportation Plan, and the project is not in KYTC's current Highway Plan.



Figure 1 – Project Area

Project Purpose

The existing conditions and constraints were identified through discussions with the Project Team.

The purpose of the project is to alleviate future congestion in this area of Oldham County and provide better access to OCEDA's Oldham Reserve Development south of I-71 and west of KY 53.

Alternatives

The following alternatives for the interchange configuration were evaluated, including the combined costs for design, right-of-way acquisition, utility relocation, and construction for the build alternatives:

- Do Nothing.
- Traffic System Management (TSM) improvements and Spot Improvements.
- Alternative 1 construct a Standard Diamond interchange at the new overpass location.
- Alternative 2 construct a Collector Distributor system with the KY 53 interchange.
- Alternative 3 improve the existing roadways in the area (i.e. no interchange). The roads would be Commerce Parkway, Allen Lane and New Moody Lane.

 Alternative 4 – construct a new road between KY 393 and New Moody Lane south of I-71 (i.e. no interchange).

		Traffic Volumes					
Alternatives	I/C Spacing	КҮ 393↓	I-71↓	Allen Lane ↑	New Moody W ↑	New Moody E ↓	KY 53↓
Standard Diamond		✓	\checkmark	✓	\checkmark	✓	✓
C-D	✓	 ✓ 	\checkmark	√	✓	✓	✓
Existing Roads	√	 ✓ 		✓	\checkmark		
New Road	✓	 ✓ 		\checkmark	\checkmark		 ✓

The following table shows the benefits to I-71 and the project area roads:

Key: I/C Spacing – Alternative meets or exceeds current AASHTO criteria.

 \downarrow - A decrease in traffic volume on the road would be a benefit.

 \uparrow - An increase in traffic volume on the road would be a benefit.

Conclusion

After a careful review and consideration of the existing conditions, the cost and benefits, and constraints of the build alternatives, the Project Team acknowledges that an interchange at the proposed overpass alleviates future traffic congestion in this area of Oldham County and best serves the purpose and need. The reasons to advance an interchange are as follows:

- Improving the existing roads or adding a new road does not improve KY 53 as much as an interchange.
- The cost of constructing an interchange is less than other alternatives because of Right-of-Way costs.

Advancing an alternative will require further detailed design and analysis, including a full Interchange Justification Study (IJS); National Environmental Policy Act (NEPA) analysis and documentation; preliminary detail engineering and design; and public involvement before approval by Federal Highway Administration (FHWA).

1.0 INTRODUCTION

1.1 Purpose of the Study

The purpose of this feasibility study is to assist the Kentucky Transportation Cabinet (KYTC) in evaluating the feasibility of providing a new interchange on I-71 in Oldham County. The study examines possible alternatives, including two interchange configurations at a proposed overpass location (see Figure 1).



Figure 1 – Project Area

1.2 Project Background

The Kentucky Transportation Cabinet's Six Year Highway Plan (FY 2007 – 2012) established the overpass project:

OLDHAM COUNTY

Item No. & Parent No.		Description	Funding	Phase	Year	Amount
2006	05-8201.01	Construct new I-71 overpass with	SP	D	2007	\$2,000,000
		approaches from Commerce Pky to	SP	R	2008	\$1,000,000
		Peak Road. (06CCR)	SP	U	2008	\$1,000,000
			SP	С	2008	<u>\$11,000,000</u>
					Total	\$15,000,000

The overpass establishes connectivity in LaGrange across I-71. The overpass will carry a four lane typical section with a raised 12 ft. median barrier and 9 ft. combined use paths on both sides.

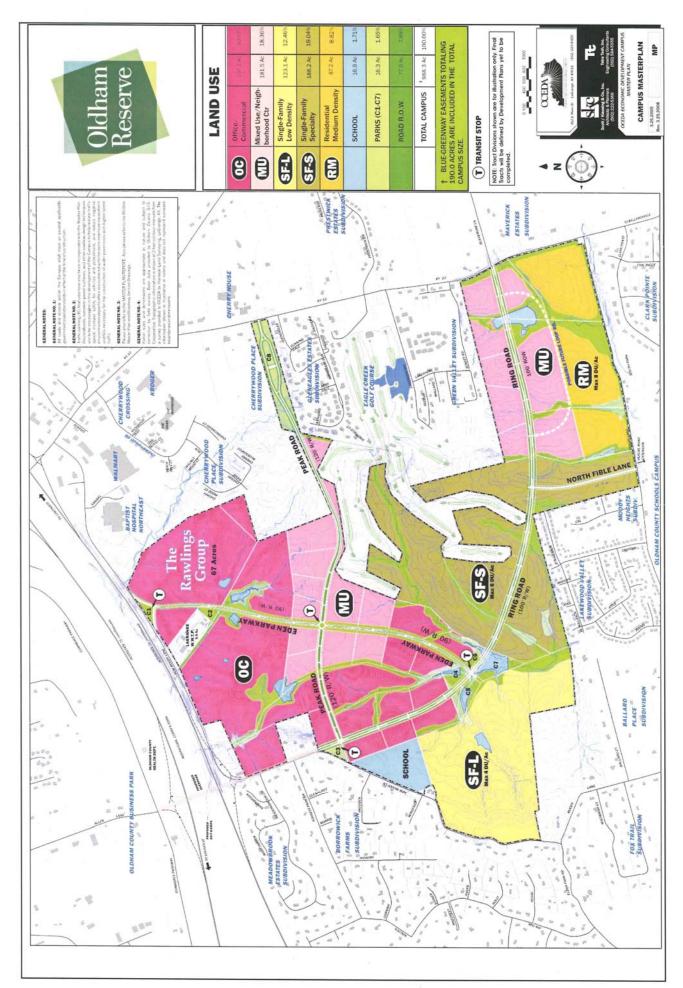
The Oldham County Economic Development Authority (OCEDA) is developing a nearly 1,000 acre mixed use campus to the south of I-71, east of the overpass and west of KY 53 (see exhibit on the next page). OCEDA has asked the KYTC to study improved access opportunities to this development.

1.3 **Project Purpose and Need**

The purpose of the project is to alleviate traffic congestion in the project area, specifically on KY 53.

The need for the project is supported by the following facts:

- High accident rates occur along KY 53 and I-71 in the vicinity KY 53 interchange (Exit 22) area.
- Current level of Service (LOS) in the project area is C or worse on all of the major roads in the project area. Design year LOS analysis shows a majority of the roads at unacceptable conditions (D, E, F).
- Traffic backups occur frequently on the KY 53 northbound exit ramp.
- Improved access is needed to Oldham Reserve.



2.0 EXISTING CONDITIONS

2.1 Project Location

The project is located in east central Oldham County, Kentucky near the city of LaGrange. The project area centers on a proposed overpass of I-71 at mile point 20.6. The I-71 corridor has the following adjacent interchanges:

- Exit 17 (KY 146) Buckner
- Exit 18 (KY 393) Buckner
- Exit 22 (KY 53) LaGrange and Ballardsville

The local roads of Commerce Parkway to the north and New Moody Lane to the south will be connected by the proposed overpass. Allen Lane is the local road between Commerce Parkway and KY 146 that connects the overpass to the north (see Figure 1, *Project Area on Page 3*).

2.2 Roadway Characteristics

The number of lanes and functional classification of the major roadways in the project area is as follows:

- <u>I-71:</u> Urban Interstate; four lanes throughout the project area.
- <u>KY 146:</u> Rural Minor Arterial; two lanes from I-71 to KY 53.
- <u>KY 393:</u> Rural Minor Arterial; newly constructed to three lanes south of I-71 (widening to three lanes north of I-71 is planned).
- KY 53: Urban Principle Arterial; five lanes south of I-71, and four lanes north of I-71.
- <u>Commerce Parkway</u>: Local two lane road north of I-71 between KY 393 and KY 53.
- <u>New Moody Lane</u>: Local two lane road south of I-71 from overpass location to KY 53.

2.3 Traffic Conditions

The following paragraphs provide summaries of traffic information. The traffic analysis was prepared by Qk4 for a base year of 2015 and a horizon year of 2035. Existing traffic volumes (year 2008) were obtained from the KYTC Highway Information System (HIS) database. Future year build and no-build traffic volumes were predicted using a travel demand model developed by Wilbur Smith for the Oldham County Economic Development Campus Traffic Impact Study, which included the proposed overpass.

A traffic volume/roadway capacity analysis was performed for this project, using existing and future year modeled traffic volumes.

• Interstate 71 currently has traffic volumes averaging 59,900 vehicles per day (vpd) in the project area, which are projected to increase to 88,000 vpd by the year 2035 (see *No Build Traffic Volumes and Level of Service*, in Appendix B). It should be noted that there

are currently no plans in either the KYTC's Highway Plan or KIPDA's Long Range Plan to add any additional lanes to I-71.

- KY 53 currently has traffic volumes averaging 17,300 vpd south of I-71 and 15,200 vpd north of I-71. Traffic volumes are projected to increase to 24,600 vpd south of I-71 and 42,300 vpd north of I-71 by the year 2035. Currently KIPDA lists several projects in their Long Range Plan concerning KY 53. These projects include widening both north and south of I-71 combined with an access management study to improve traffic flow and congestion.
- KY 146 currently has traffic volumes averaging 12,400 vpd. Traffic volumes are projected to increase to 25,900 vpd by the year 2035. Currently KIPDA has projects listed in their Long Range Plan to widen KY 146 to four lanes throughout most of the study area.
- KY 393 currently has traffic volumes averaging 7,720 vpd in the project area, which are projected to increase to 19,800 vpd by the year 2035. Currently KIPDA has projects listed in their Long Range Plan to widen KY 393 to three lanes throughout most of the study area.

2.4 Level of Service

"Level of service" is defined as a qualitative measure of operational conditions, and the motorists' perception of those conditions. The conditions are usually defined in terms such as speed, travel time, maneuverability, delay, and comfort and convenience. The letters "A" through "F" designate the six levels of service. LOS A represents the best operating conditions (*i.e.*, free flow conditions), while LOS F defines the worst (*i.e.*, severe congestion). According to the national standards, the lower levels of service (*i.e.*, D, E, and F) are unacceptable for safe and efficient operation. The lower levels generally involve unstable traffic flows, and drivers have little freedom to maneuver.

The LOS analysis performed on study area roadways indicates the 2008 LOS ranges from B to D (see Table 1, *Level of Service Comparison*, page 8). By the year 2035, LOS is predicted to generally decrease, resulting in a range from C to F. The increasing traffic volume combined with a decreasing level of service will eventually cause regularly occurring peak hour congestion and its associated delays in accessing businesses, along with increased driver frustration and the likelihood for higher crash rates.

Typically, LOS D is considered the minimum acceptable in urban areas. LOS E and F are, therefore highlighted yellow and orange, respectively.

Route	From	То	2008 Existing		20 No-E		203 No-B	
I-71	West	KY 146	59900	D	69400	Е	88000	F
I-71	KY 146	KY 393	58200	D	62600	D	75800	E
I-71	KY 393	KY 53	57400	D	60500	D	69400	Е
I-71	KY 53	East	38600	С	44600	С	58800	D
KY 146	West	I-71	8810	С	19200	F	25900	F
KY 146	I-71	KY1817	12100	С	16200	D	24900	F
KY 146	KY 1817	KY 393	12400	С	12400	С	21200	F
KY 146	KY 393	KY 393	11600	С	12100	С	21600	F
KY 146	KY 393	KY 2854	8130	С	6400	С	11200	С
KY 146	KY 2854	KY 53	9570	С	7200	С	10700	С
KY 146	KY 53	East	5740	С	3100	В	6300	С
KY 393	South	KY 2856	5250	С	12100	С	16100	D
KY 393	KY 2856	I-71	6710	С	14300	D	19800	F
KY 393	I-71	KY 146	7720	С	9100	С	18800	F
KY 393	KY 146	North	4060	В	5200	С	12300	С
KY 53	South	KY 2856	6350	С	7600	С	23200	F
KY 53	KY 2856	Connector	9030	С	12000	С	25200	F
KY 53	Connector	I-71	17300	С	14400	С	24600	С
KY 53	I-71	KY 146	15200	С	33000	E	42300	F
KY 53	KY 146	North	9200	С	18700	С	28000	D

Table 12008, 2015 and 2035 Traffic Conditions

2.5 Existing Interchange Geometry

The following table shows that the existing interchanges in the study area meet AASHTO criteria for acceleration and deceleration lengths in every case except the northbound entrance ramp at Exit 17 (KY 146).

Table 2 Existing Interchange Geometry

Ramps	Exit Ramp Storage Length	Deceleration Actual (Recommended)	Acceleration Actual (Recommended)
Exit 17 NB	1800 ft.	520 (340)	300 (1230)**
Exit 17 SB	2350 ft.	560 (550)	600 (580)
Exit 18 NB	2005 ft.	730 (340)	750 (580)
Exit 18 SB	1760 ft.	700 (340)	*2000 (580)
Exit 22 NB	1500 ft.	540 (340)	800 (580)
Exit 22 SB	1317 ft.	560 (340)	800 (580)
*Derellel Ture			
*Parallel Type			
**Substandard (3	crashes, 6800 ADT)		

2.6 Crash Analysis

Safety along the major roadways in the project study area was analyzed using accident report data in a crash analysis. A crash analysis is a mathematical tool for finding roadway sections with abnormally high crash rates and, therefore, sections with potentially correctable hazards to traffic safety. Historical

crash data from the three-year period January 2005 – December 2007 was used to identify roadway sections with abnormally high crash rates. The traffic crash analysis indicates five roadway sections in the project study area are experiencing high crash rates. Table 3, *Crash Analysis Summary*, lists the high crash locations for the project area. When the critical rate factor (CRF) is higher than 1.0, statistically the rate of crashes is higher than normal as compared to other roads with the same functional classification and traffic volumes. The complete analysis is shown in Appendix A.

Route	Begin	End	Location Description	CRF
	Milepoint	Milepoint		
KY-53	5	5.685	South of Gleneagles Way to Zhale Smith Rd	1.0-1.5
KY-53	6.296	7.055	I-71 Overpass to KY 146	1.5-2.5
KY-53	5.685	6.296	Zhale Smith Road to I-71 Overpass	2.5-3.0
KY-146	6.829	7.419	KY 1817 to KY 393 North	1.0-1.5
I-71	21.65	22.15	South and North of the KY 53 Underpass	1.0-1.5

Table 3Crash Analysis Summary

3.0 PROJECT TEAM INPUT

3.1 Project Team Meetings

The I-71 Interchange study project team met three times: 1) prior to the initiation of this study; 2) to review alternatives and; 3) to make a recommendation. The meetings were documented with meeting minutes. A brief summary of the major topics discussed at each meeting follows:

- PTM #1 Kickoff Meeting Summary (March 10, 2008)
 - Evaluation Options
 - Existing Conditions
 - Proposed Projects
 - o Crash History
 - o Traffic Data
 - o Constraints
- PTM #2 Summary (Review of Alternatives) (August 8, 2008)
 - Traffic analysis review of each alternative including volume, LOS, merge, diverge and weave
 - Preliminary interchange layouts
 - Standard diamond
 - Collector Distributor
 - o Interchange layouts eliminated from detailed study
 - Single Point Urban
 - Split Diamond
- Discussion
 - Show benefits of alternatives
 - o Document assumptions made
 - Study I-71 widening
 - o Study improvements to existing KY 53 interchange
 - Check AASHTO criteria met at existing interchanges
- PTM #3 Summary (Review Alts. / Make Recommendation) (August 29, 2008)
 - o Oldham County provided the most recent plan for the Oldham Reserve
 - Determine when the interstate needs to be widened after the interchange is constructed
 - o I-71 LOS's go from E & F (4 lanes) to C & D (6 lanes)
 - Identify projects (I-71 Widening) that need to be done with or without a new interchange.
 - o Other improvement concepts discussed:
 - 1/2 Interchange originally eliminated at the Kickoff Meeting (PTM #1)
 - Parallel route south of I-71 construction of a new road between KY 393 and New Moody

• FHWA will review the report, make comments and decide whether to grant Engineering and Operational approval pending completed IJS.

The above meetings were documented with minutes (see Appendix I).

3.2 Constraint Identification

- Commerce Parkway Possible Relocation
- Meadowbrook Subdivision
- New Moody Lane
- Widening of I-71 to Inside (Currys Fork)
- Spacing with LaGrange Exit
- LaGrange Treatment Plant
- Business on east side of Commerce Parkway
- Major utilities crossing the Study Area (gas, power)
- Proposed bike and pedestrian facility on north side of Commerce Parkway

4.0 STUDY ALTERNATIVES CONSIDERED

4.1 Transportation System Management

Transportation System Management (TSM) involves relatively low-cost improvements, but effective in nature, that can be quickly implemented through roadway maintenance activities. TSM improvements generally refer to such things as signing at critical locations, traffic signals at intersections, lighting, and simple roadway improvements such as pavement striping, removing vegetation to improve visibility, or improving the radius of a street corner. No TSM options are found to be sufficient to significantly improve the interstate connectivity in the study area. However, because of limited access management on KY 53 (Oldham County is striving to improve), TSM improvements, such as signal timing adjustments, should be investigated as possible short-term safety projects.

4.2 Spot Improvements

 The only spot improvement identified that could be implemented to improve traffic flow and safety is the installation of dual left turn lanes on the NB Exit Ramp from I-71 to KY 53. The construction cost is estimated to be less than \$250,000.

4.3 Independent System Improvements

The following projects have been identified as recommended improvements in the area:

I-71, Six Lanes – Current ADT's on I-71 are between 57,400 and 59,900. Assuming a 10% K Factor, a 65/35 D Factor and that interstate capacity is 2200 vph per lane, I-71 would need to be 3 lanes in 2009 south of KY 146; in 2013 between KY 146 and KY 393 and in 2015 between KY 393 and KY 53. An estimated cost for this improvement is as follows:

Construction	\$31,500,000
R/W	100,000
Utility	100,000
Engineering	300,000
Total	\$34,700,000

 Two Lane Exit Ramp to KY 53 (I-71 NB to KY 53) – Current ADT on the ramp is 13,400. The projected No-Build ADT's for 2015 and 2035 are 16,600 and 20,100 respectively. Assuming a 10% K Factor and that ramp capacity is 1,800 for speeds less than 20 mph (i.e. ramp backs up) the ramp would need to be two lanes in 2023. If a Standard Diamond interchange at the new overpass location is constructed, however, the traffic model shows that a one lane ramp is sufficient.

Construction	*\$1,000,000
R/W	100,000
Utility	100,000
Engineering	100,000
Total	\$1,300,000

*Cost includes dual left turn lanes at KY 53.

 KY 53 – widening KY 53 through the interchange to accommodate three lanes in each direction and left turn lanes would produce operation at a low LOS D (borderline LOS E). With the heavy turn movements at some of the existing intersections, it would actually operate much worse than this, as current traffic (15,200) is less than half of the projected volume (42,300) and there are concerns now.

Construction	\$3,500,000
R/W	1,000,000
Utility	1,000,000
Engineering	400,000
Total	\$5,900,000

4.4 Design Alternatives

A do-nothing and four build alternatives were evaluated for this Feasibility Study. The five alternatives are described below. The cost estimates are in 2008 dollars.

Do Nothing Alternative. The Do Nothing Alternative involves only routine roadway maintenance. No action will be taken to construct an interchange or improve existing roads. This option will be referred to as appropriate for baseline comparisons throughout the decision making process (see Appendix C).

Alternative 1 – Construct a Standard Diamond interchange at the overpass location. Appendix D shows a preliminary sign layout. New Moody Lane is relocated to opposite the entrance to Meadowbrook Subdivision to meet access control requirements (>300 ft.). The ramp lengths, acceleration and deceleration lengths meet AASHTO criteria. The spacing between this location, which would be Exit 20 (overpass at MP 20.6) and the KY 53 interchange (Exit 22) is not as desirable as Alternative 2 (C-D). The desirable distance is 3 miles due to the rural character of the project area. This alternative improves emergency access to the hospital in LaGrange. The cost estimate is as follows:

Construction	\$4,900,000
Right of Way	1,000,000
Utilities	350,000
Engineering	500,000
TOTAL	\$6,650,000

Alternative 2 - Construct a one lane Collector-Distributor system from south of the overpass location to north of KY 53. Appendix E shows a preliminary sign layout. New Moody Lane is relocated to opposite the entrance to Meadowbrook Subdivision to meet access control requirements (>300 ft.). The ramp lengths, acceleration and deceleration lengths meet AASHTO criteria. There is not a spacing problem, since the interchanges are combined. Existing Exit 22 becomes Exit 20B. This alternative improves emergency access to the hospital in LaGrange. The cost estimate is a follows:

Construction	\$22,600,000
Right of Way	3,000,000
Utilities	500,000
Engineering	2,300,000
TOTAL	\$28,400,000

Alternative 3 – Improve the existing roads in the project area (i.e. no interchange at the overpass). Widening of Commerce Parkway, Allen Lane and New Moody Lane to 2 lanes in each direction between KY 393 and KY 53 is an alternative to constructing an interchange. Vehicles would use the KY 393 interchange (Exit 18) to access Oldham Reserve and downtown LaGrange. The cost estimates are as follows:

COMMERCE Fairway	
Construction Right of Way Utilities Engineering TOTAL	\$11,800,000 1,500,000 500,000 <u>1,200,000</u> \$15,000,000
Allen Lane	
Construction Right of Way Utilities Engineering TOTAL	\$3,500,000 250,000 100,000 <u>250,000</u> \$3,100,000
<u>New Moody Lane</u>	
Construction Right of Way Utilities Engineering TOTAL	\$4,900,000 200,000 500,000 <u>500,000</u> \$6,100,000
TOTAL COST =	\$24,200,000

Commerce Parkway

Note: See Appendix F for Traffic Volumes, LOS and Merge, Diverge and Weave Analysis.

Alternative 4 – construct a new road between KY 393 and New Moody Lane on the south side of I-71 (i.e. no interchange at the overpass). This alternative would require New Moody Lane to be widened also. Appendix G shows a preliminary layout of the proposed road. The major utilities identified as constraints and three subdivisions are impacted. The cost estimate is as follows:

Construction	\$8,300,000
Right of Way	30,000,000
Utilities	2,000,000
Engineering	800,000
TOTAL	\$41,100,000

4.4.1 Alternatives Eliminated from Detail Study

- Half Interchange Eliminated at kickoff meeting (further discussed at PTM #3; IJS may choose to reconsider).
- Single Point Urban Eliminated at kickoff meeting.
- Split Diamond KY 53 intersection capacity LOS F.
- Flopped Diamond R/W impacts.

4.5 Alternative Comparison

The following alternative comparison is focused on the increase or decrease in traffic volumes for the road network in the area (\uparrow signifies an increase; \downarrow signifies a decrease).

KY 393 North of I-71 (Like to see a decrease on this segment.)		
	<u>2015</u>	<u>2035</u>
Diamond	↓ 1500	↓ 3100
C-D	↓ 400	↓ 2600
Existing Roads	↑ 2200	↑ 2700
New Road	↓ 2600	↓ 6400

I-71 between overpass and KY 53 (Like to see a decrease on this segment.)		
	2015	2035
Diamond	↓ 8700	↓ 4500
C-D	↓ 36100	↓ 17900
Existing Roads	↑ 2300	↑ 1500
New Road	↑ 1700	1800

Allen Lane (Like to see an increase of traffic.)		
	<u>2015</u>	2035
Diamond	↑ 4500	↑ 4500
C-D	↑ 3400	↑ 3700
Existing Roads	↑ 2700	↑ 900
New Road	1 3000	1800

Commerce Parkway (Like to see an increase between Allen Lane and downtown.)			
	<u>2015</u>	<u>2035</u>	
Diamond	↑ 600	↑ 2400	
C-D	↑ 900	↑ 4500	
Existing Roads	↑ 1500	↑ 3000	
New Road	↑ 200	↓ 3500	

New Moody Lane West of Rawlings (Like to see an increase.)		
	<u>2015</u>	2035
Diamond	↑ 5900	↑ 4800
C-D	↑ 6200	↑ 4500
Existing Roads	↑ 1700	↑ 2900
New Road	1 2300	↑ <u>4</u> 800

New Moody Lane east of Rawlings (Like to see a decrease.)		
<u>2015</u>	<u>2035</u>	
↓ 3500	No change	
↓ 3300	↑ 400	
↑ 900	↓ 2000	
↑ 2200	↓ 600	
	<u>2015</u> ↓ 3500 ↓ 3300 ↑ 900	

KY 53 North (Like to see a decrease in traffic using KY 146 or Commerce Parkway to get to downtown.)		
	<u>2015</u>	<u>2035</u>
Diamond	↓ 4600	↓ 1700
C-D	↓ 3500	↓ 4200
Existing Roads	↓ 2300	↑ 1500
New Road	↓ 4100	↓ 1100

KY 53 South (Like to see a decrease.)		
	<u>2015</u>	2035
Diamond	No change	↓ 5700
C-D	No change	↓ 5400
Existing Roads	↑ 3400	↓ 6500
New Road	↑ 7300	↓ 2500

The following table compares the projected ramp volumes in 2015 of an interchange at the overpass to the projected 2015 KY 393 interchange ramp volumes.

Ramp Volumes: New interchange vs KY 393			
	New	<u>KY 393</u>	$\underline{\Delta}$
NB off	5400	4500	+ 900
NB on	1100	3900	-2800
SB off	1100	1900	- 800
SB on	5200	3900	+1300

The following table shows the benefits to I-71 and the project area roads.

Table 4 Benefits Matrix	
-------------------------	--

		Traffic Volumes												
Alternatives	I/C Spacing	KY 393↓	I-71↓	Allen Lane ↑	New Moody W ↑	New Moody E ↓	KY 53↓							
Standard Diamond		√	✓	√	~	✓	✓							
C-D	√	✓	~	~	✓	✓	✓							
Existing Roads	~	~		~	✓									
New Road	✓	✓		✓	✓		✓							

Key: I/C Spacing – Alternative meets or exceeds current AASHTO criteria.

 \downarrow - A decrease in traffic volume on the road would be a benefit.

 \uparrow - An increase in traffic volume on the road would be a benefit.

5.0 CONCLUSIONS

5.1 Recommendation

After a careful review and consideration of the existing conditions, the cost and benefits, and constraints of constructing either an interchange, improving existing roads or constructing a new road **the Project Team acknowledges that an interchange at the proposed overpass location alleviates future traffic congestion in this area of Oldham County, and best serves the purpose and need.** The interchange could be a C–D, Standard Diamond or another configuration (such as a partial interchange) following more detailed traffic and design analysis. The Project Team also recognizes widening of I-71 and KY 53 are needed with or without an interchange.

Draft recommendation based on:

- Existing conditions improving the existing roads or constructing a new road did not reduce the amount of traffic on the KY 53 NB exit ramp that presently backs into I-71.
- Constraints there are not major impacts on the identified constraints for the project except the impact to Meadowbrook Subdivision in Alternative 4
- Benefits the C-D is the only alternative that met the project purpose in every case; the standard diamond met all but one.
- Operations the C-D did not have spacing issues, where the standard diamond did.
- Cost the Standard Diamond interchange alternative cost (\$6.5 million) is significantly less than the other alternatives. The C-D is slightly more than improving the existing roads.

5.2 Next Steps

An I-71 interchange at the proposed overpass south of LaGrange is not in the current Long Range Plan.

Since this study shows that an interchange is feasible, KYTC or Oldham County would need to introduce it into the KIPDA Long Range Plan.

5.2.1 Interchange Justification Study Analysis

The Intermodal Surface Transportation Efficiency Act of 1991 (ISTEA) contains requirements for planning a proposed interchange to the existing Interstate Highway system. These requirements are implemented in FHWA policy and through Federal regulation located in 23 CFR part 450. The policy for *Additional Interchanges to the Interstate System* contains eight points that must be taken into consideration. This section provides a preliminary overview of each policy point as it relates to this project.

Policy Statement No. 1: Existing Facilities Capability

"It is demonstrated that the existing interchanges and/or local roads and streets in the corridor can neither provide the necessary access, nor be improved to satisfactorily accommodate the design-year traffic demands while at the same time providing the access intended by the proposal. " The existing interchanges in the area (I-71/KY 146; I-71/KY 393; I-71/KY 53) and the existing local roads (Allen Lane, Commerce Parkway, New Moody Lane) could be improved to handle more capacity. The planning level traffic analysis does show that an interchange added to the system takes some traffic off already congested roads. Providing a new interchange at the proposed overpass location would provide improved and more direct access to the interstate network for Oldham Reserve. Emergency access to the hospital is also improved.

Policy Statement No. 2: Transportation System Management

"All reasonable alternatives for design options, location and transportation system management type improvements (such as ramp metering, mass transit, and HOV facilities) have been assessed and provided for, if currently justified, or provisions are included for accommodating such facilities if a future need is identified."

Transportation System Management (TSM) type improvements have not been discussed in detail for this location. TSM improvements, such as signal timing adjustments, should be investigated as possible short-term safety projects. Limited mass transit is provided for in the study area, and improved access to I-71 with an interchange would improve the transit service routes and options, including school bus routes. HOV lanes are not provided on any Louisville urban area interstates. An additional interchange in Oldham County would appear to not limit future TSM options if required.

Policy Statement No. 3: Operational Analysis

"The proposed access point does not have a significant adverse impact on the safety and operation of the Interstate facility based on an analysis of current and future traffic. The operational analysis for existing conditions shall, particularly in urbanized areas, include an analysis of sections of Interstate to and including at least the first interchange on either side. Crossroads and other roads and streets shall be included in the analysis to the extent necessary to assure their ability to collect and distribute traffic to and from the interchange with new or revised access point."

The traffic operational analysis has been performed for the interchange alternatives, and it included the interchanges to the south (KY 146, KY 393), to the north (KY 53) and the local roads within the study area. The operational analysis illustrates that a proposed standard diamond interchange is tightly spaced with the KY 53 interchange and is not the most desirable from an operational standpoint; but, adding an interchange as a Collector-Distributor system with the KY 53 interchange provides better safety and operation compared to a standard diamond.

The operational analysis shows that the local streets would be able to effectively collect and distribute traffic to and from either interchange configuration.

Policy Statement No. 4: Access Connections and Design

"The proposed access connects to a public road only and will provide for all traffic movements. Less than "full interchanges" for special purposes access for transit vehicles, for HOVs or into park and ride lots may be considered on a case-by-case basis. The proposed access will be designed to meet or exceed standards for Federal-aid projects on the Interstate system."

The proposed interchange will connect to public roads: Commerce Parkway, New Moody Lane, Allen Lane and the proposed Oldham Reserve exterior road (which connects to KY 53).

As noted in the meeting summaries provided in this report, a half interchange was discussed and eliminated from detailed study (if an interchange is concluded to be feasible at this location, a "less

than full interchange" could be considered during this phase of the project). Projected traffic volumes show that movements to and from I-71 to the south are much higher than those from and to the north. A partial interchange would provide some relief to the currently congested interchange at KY 53 as does a full interchange, but a partial interchange would not fully satisfy the purpose and need for the project.

The design of the recommended partial interchange would need to meet or exceed current design standards for Federal-aid projects on the Interstate System.

Policy Statement No. 5: Transportation and Land Use Plans

"The proposal considers and is consistent with local and regional land use and transportation plans."

The interchange is not in KIPDA's *Horizon 2030* LRTP. Current land use plans in Oldham County are consistent with a proposed interchange. Oldham County Planning Commission does have a Major Thoroughfare Plan published in December 2003. This location was not identified as a possible new interchange with I-71. Oldham County also has a comprehensive plan – Outlook 2020 – adopted in February 2002.

Policy Statement No. 6: Comprehensive Interstate Network Study

"In areas where the potential exists for future multiple interchange additions, all request for new or revised access are supported by a comprehensive Interstate network study with recommendations that address all proposed and desired access within the context of a long-term plan."

The only proposed new interchange with I-71 in Oldham County's Major Thoroughfare Plan is with KY 1694 (south of this project and outside the study area).

Policy Statement No. 7: Coordination with Transportation System Improvements

"The request for a new or revised access generated by new or expanded development demonstrates appropriate coordination between the development and related or otherwise required transportation system improvements."

Inclusion of the Oldham County Planning Commission Director; OCEDA Director and KIPDA as part of the project team demonstrates early coordination efforts. Further documented coordination would be a part of the IJS. The proposed project would provide benefit to continued development plans for Oldham Reserve and is being coordinated with other planned transportation improvements.

Policy Statement No. 8: Status of Planning and NEPA

"The request for new or revised access contains information relative to the planning requirements and the status of the environmental processing of the proposal."

The planning process and planning objectives, herein, were implemented to advance the National Environmental Policy Act (NEPA) and Interchange Justification Study (IJS) requirements of the FHWA, should this project be advanced. The planning level analysis concludes the interchange would be beneficial to area traffic and not harmful to the interstate network. Regarding the NEPA process, no significant impacts are anticipated with an interchange; therefore, either a Categorical Exclusion or an Environmental Assessment/Finding of No Significant Impact document should be appropriate.

5.2.2 NEPA Requirements

The following environmental issues are likely to require consideration for this project.

- Historic, Archaeological, and Cultural Resources
- Aquatic Resources
- Wetlands and Ponds
- Threatened and Endangered Species
- Hazardous Materials Concerns
- Air Quality
- Traffic Noise
- Community Facilities
- Environmental Justice

5.2.3 Preliminary Engineering

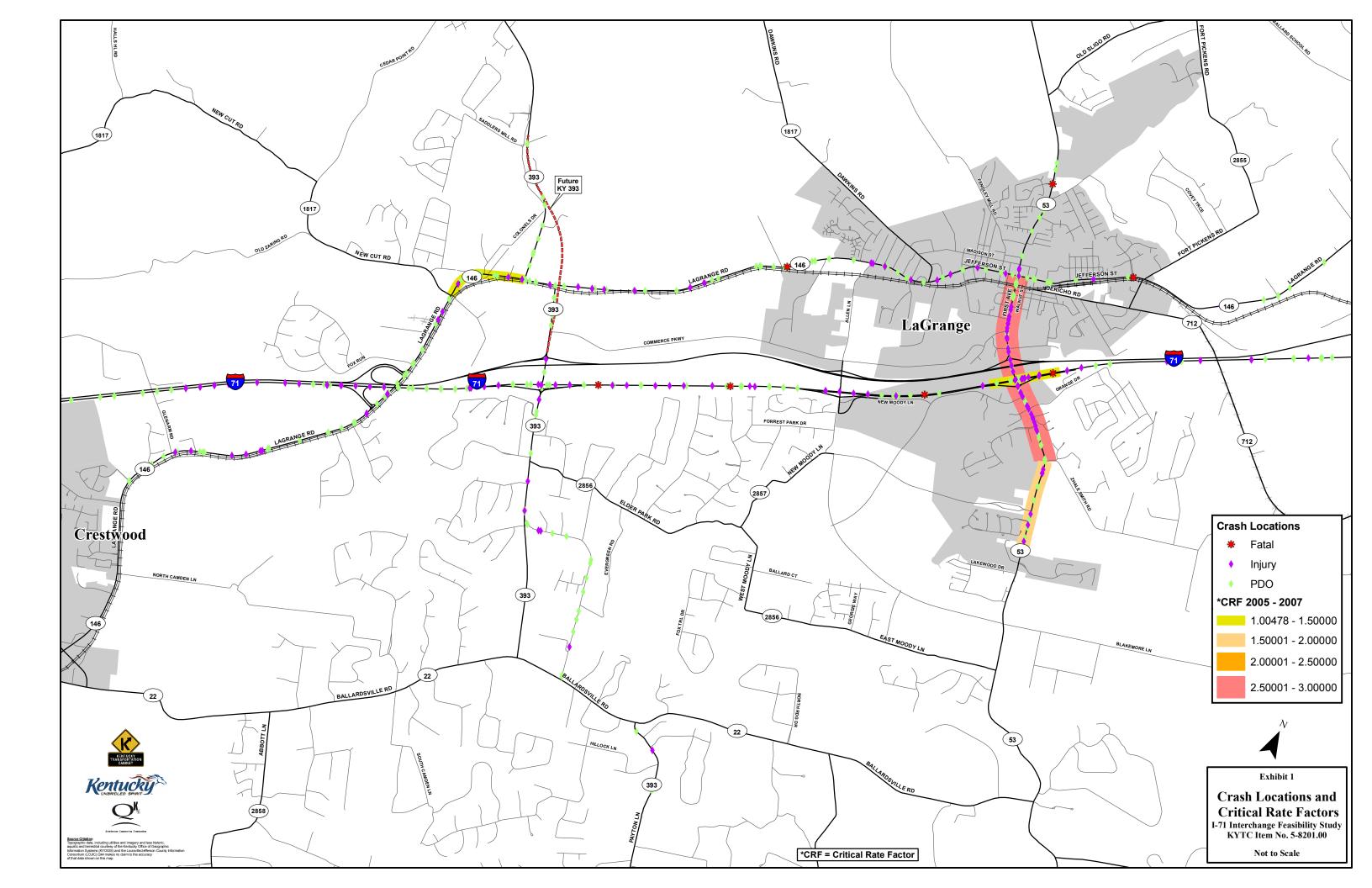
A more detailed interchange design would require the following elements:

- Update mapping with digital terrain information
- Finalize typical sections
- Finalize geometry (compatible with all recommendations)
- Study drainage
- Conceptual Traffic Management Plan
- Establish preliminary R/W
- Refine cost estimate
- Select recommended alternative/DES

5.2.4 Public Involvement

Advancement of the project would necessitate a public involvement component. Public input and comments would need to be included with the NEPA document.

Appendix A Crash Analysis



C		Rate Analys Iway Segment	sis			
County		Oldham				
Route		Kentucky 53		-		
Milepoint Range	5.685	to	6.296	•		
Date		March 3, 2008		-		
Completed By		J Lukat		-		
Critical Rate Analysis and Crash History						
Total Number of Crashes at Location						148
Total Number of Nighttime Crashes at Location						20
Traffic Volume at Location						17,900
Date Range of CRASH Query			Janua	ry 1, 2005	to	December 31, 2007
Roadway Typical Section					_	4-Lane, Undivided
Roadway Classification						Urban
Critical Rate Analysis						
Statewide Average Crash Rate						4.320
Critical Rate						6.279
Critical Rate Factor						2.956
Is the Critical Rate Factor greater than or equal to 1.0)?					Yes
Nighttime Critical Rate Analysis						
Nighttime Statewide Average Crash Rate						5.350
Critical Rate						10.837
Nighttime Critical Rate Factor						1.362

No

Nighttime Critical Rate Factor

County		Rate Analys Iway Segment Oldham	sis			
Route		Kentucky 146				
Milepoint Range	6.829	to	7.419			
Date		March 3, 2008				
Completed By		J Lukat				
Critical Rate Analysis and Crash History						
Total Number of Crashes at Location						30
Total Number of Nighttime Crashes at Location						2
Traffic Volume at Location						12,100
Date Range of CRASH Query			Janua	ry 1, 2005	to	December 31, 2007
Roadway Typical Section			-		_	Two Lane
Roadway Classification						Rural
Critical Rate Analysis						
Statewide Average Crash Rate						2.220
Critical Rate						3.657
Critical Rate Factor						1.050
Is the Critical Rate Factor greater than or equal to 1	.0?					Yes
Nighttime Critical Rate Analysis						
Nighttime Statewide Average Crash Rate						3.790
Critical Rate						8.660
Nighttime Critical Rate Factor						0.184

No

Route Kentucky 53 Milepoint Range 6.296 to 7.055 Date March 3, 2008	County Route		Rate Analys way Segment Oldham	sis			
Date Completed ByMarch 3, 2008Critical Rate Analysis and Crash HistoryJ LukatTotal Number of Crashes at Location138Total Number of Nighttime Crashes at Location23Traffic Volume at Location16,000Date Range of CRASH QueryJanuary 1, 2005toDecember 31, 20074-Lane, UndividedRoadway Typical SectionUrbanRoadway Classification0Critical Rate Analysis4.20Statewide Average Crash Rate4.320Critical Rate Factor6.176Is the Critical Rate Factor greater than or equal to 1.0?Yes		(20(Kentucky 53	7.055			
Completed ByJ LukatCritical Rate Analysis and Crash History138Total Number of Crashes at Location23Total Number of Nighttime Crashes at Location23Traffic Volume at Location16,000Date Range of CRASH QueryJanuary 1, 2005toDecember 31, 20074-Lane, UndividedRoadway Typical SectionUrbanRoadway ClassificationUrbanCritical Rate Analysis4.320Statewide Average Crash Rate6.176Critical Rate2.524Is the Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes		6.296		/.055			
Critical Rate Analysis and Crash HistoryTotal Number of Crashes at Location138Total Number of Nighttime Crashes at Location23Traffic Volume at Location16,000Date Range of CRASH QueryJanuary 1, 2005toRoadway Typical Section4-Lane, UndividedRoadway ClassificationUrbanCritical Rate AnalysisStatewide Average Crash Rate4.320Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes			-				
Total Number of Crashes at Location138Total Number of Nighttime Crashes at Location23Traffic Volume at Location16,000Date Range of CRASH QueryJanuary 1, 2005toRoadway Typical SectionDecember 31, 2007Roadway ClassificationUrbanCritical Rate AnalysisStatewide Average Crash Rate4.320Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Completed By		J Lukai				
Total Number of Nighttime Crashes at Location23Traffic Volume at Location16,000Date Range of CRASH QueryJanuary 1, 2005toRoadway Typical SectionDecember 31, 2007Roadway Classification4-Lane, UndividedUrbanUrbanCritical Rate AnalysisStatewide Average Crash Rate4.320Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Critical Rate Analysis and Crash History						
Traffic Volume at Location16,000Date Range of CRASH Query Roadway Typical Section Roadway ClassificationJanuary 1, 2005toDecember 31, 20074-Lane, Undivided UrbanUrbanCritical Rate Analysis Statewide Average Crash Rate Critical Rate Critical Rate 	Total Number of Crashes at Location						138
Date Range of CRASH Query Roadway Typical Section Roadway ClassificationJanuary 1, 2005toDecember 31, 20074-Lane, Undivided UrbanUrbanCritical Rate AnalysisStatewide Average Crash Rate Critical Rate4.320Critical Rate Critical Rate Factor Is the Critical Rate Factor greater than or equal to 1.0?5.2524Yes	Total Number of Nighttime Crashes at Location						23
Roadway Typical Section4-Lane, UndividedRoadway ClassificationUrbanCritical Rate Analysis4-Lane, UndividedStatewide Average Crash Rate4.320Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Traffic Volume at Location						16,000
Roadway ClassificationUrbanCritical Rate Analysis4.320Statewide Average Crash Rate6.176Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Date Range of CRASH Query			Januar	y 1, 2005	to	December 31, 2007
Critical Rate AnalysisStatewide Average Crash RateCritical RateCritical RateCritical Rate FactorIs the Critical Rate Factor greater than or equal to 1.0?Yes	Roadway Typical Section						4-Lane, Undivided
Statewide Average Crash Rate4.320Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Roadway Classification						Urban
Critical Rate6.176Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Critical Rate Analysis						
Critical Rate Factor2.524Is the Critical Rate Factor greater than or equal to 1.0?Yes	Statewide Average Crash Rate						4.320
Is the Critical Rate Factor greater than or equal to 1.0? Yes	Critical Rate						6.176
	Critical Rate Factor						2.524
Nighttime Critical Rate Analysis	Is the Critical Rate Factor greater than or equal to 1.	0 ?					Yes
	Nighttime Critical Rate Analysis						
Nighttime Statewide Average Crash Rate 5.350	Nighttime Statewide Average Crash Rate						5.350
Critical Rate 10.539	0						10.539

1.450

No

Nighttime Critical Rate Factor

C		Rate Analys Iway Segment	sis			
County		Oldham				
Route		Interstate 71				
Milepoint Range	21.65	to	22.15			
Date		March 3, 2008				
Completed By		J Lukat				
Critical Rate Analysis and Crash History						
Total Number of Crashes at Location						52
Total Number of Nighttime Crashes at Location						11
Traffic Volume at Location						46,000
Date Range of CRASH Query			January	1, 2005	to	December 31, 2007
Roadway Typical Section						Interstate
Roadway Classification						Urban
Critical Rate Analysis						
Statewide Average Crash Rate						0.960
Critical Rate						1.483
Critical Rate Factor						1.393
Is the Critical Rate Factor greater than or equal to 1.0)?					Yes
Nighttime Critical Rate Analysis						
Nighttime Statewide Average Crash Rate						1.180
Critical Rate						2.584
Nighttime Critical Rate Factor						0.914

No

Cı		l Rate Analys dway Segment	sis			
County	nou	Oldham				
Route		Kentucky 53				
 Milepoint Range	5	to	5.685			
Date		March 3, 2008				
Completed By		J Lukat				
Critical Rate Analysis and Crash History						
Total Number of Crashes at Location						32
Total Number of Nighttime Crashes at Location						6
Traffic Volume at Location						8,600
Date Range of CRASH Query			Januai	ry 1, 2005	to	December 31, 2007
Roadway Typical Section					_	Two Lane
Roadway Classification						Urban
Critical Rate Analysis						
Statewide Average Crash Rate						2.610
Critical Rate						4.735
Critical Rate Factor						1.574
Is the Critical Rate Factor greater than or equal to 1.0?	,					Yes
Nighttime Critical Rate Analysis						
Nighttime Statewide Average Crash Rate						3.130
Critical Rate						9.149
Nighttime Critical Rate Factor						0.898

No

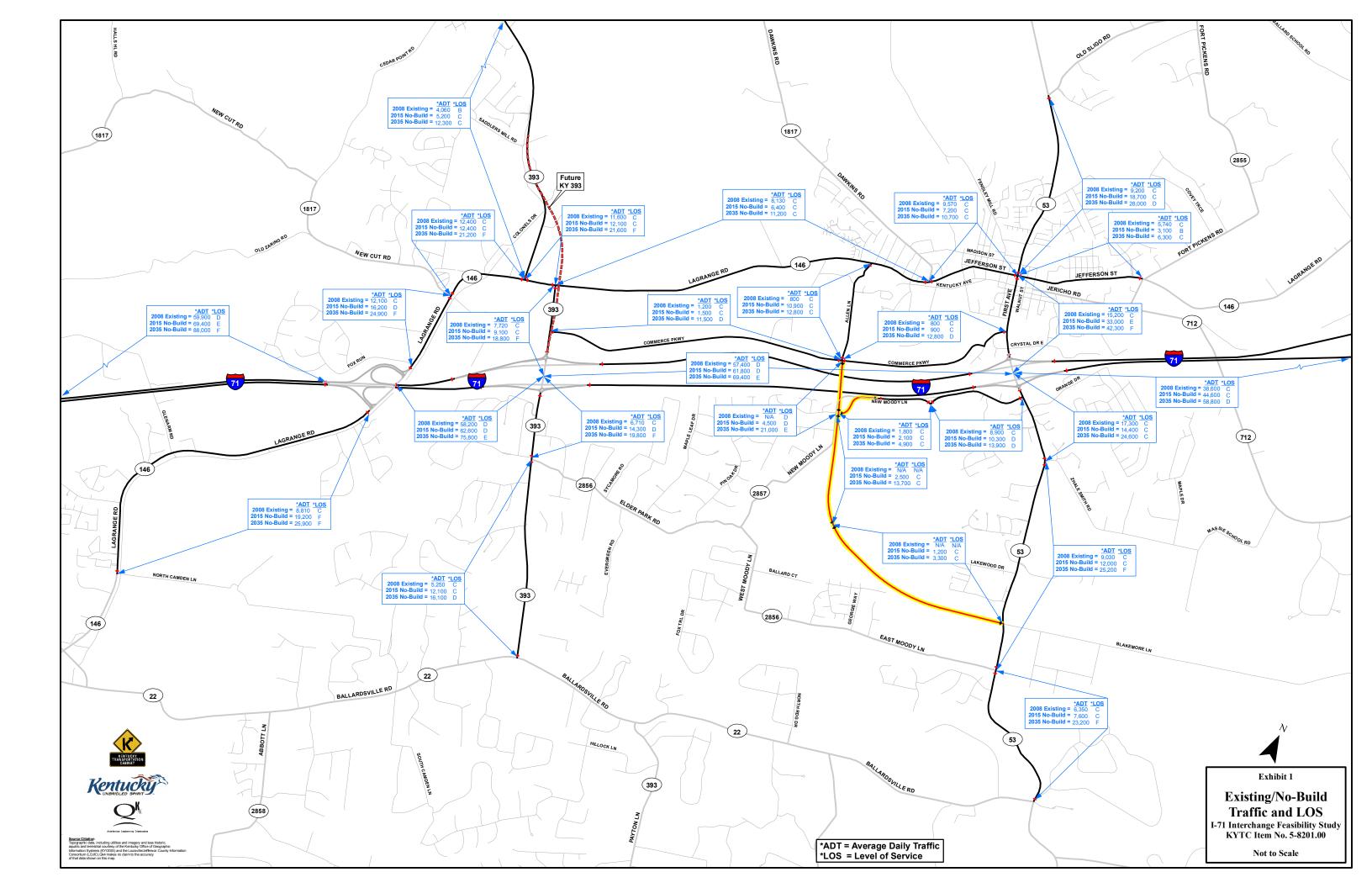
Appendix B Traffic Volumes & LOS Summary

TRAFFIC VOLUMES LOS SUMMARY EXHIBIT 1 - APPENDIX B

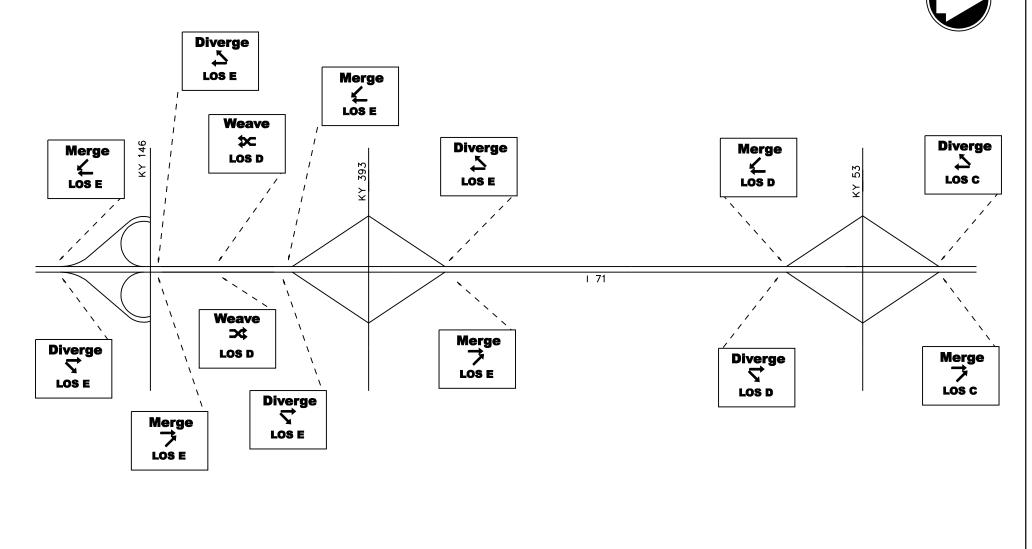
		20	08		2015					2035							35						
Route	From / To	Exis	ting	No-E	Build	Diam	ond	С	D	Wide	ning	New	Road	No-E	Build	Diamo	nd	CI	D	Wide	ning	New	Road
I-71 (1)	West of KY 146	59900	D	69400	E	69400	Е	69400	E	69400	E	69400	E	88000	F	88000	F	88000	F	88000	F	88000	F
I-71 (3)	between KY 146 and KY 393	58200	D	62600	D	64500	D	61500	D	63400	D	63200	D	75800	E	76300	E	77900	F	76600	F	76400	E
I-71 (5)	between KY 393 and Proposed Interchange	57400	D	60500	D	61200	D	60800	D	59500	D	63500	D	69400	E	71500	E	72300	E	63300	D	67600	E
I-71 ML (8)	between Proposed Interchange and KY 53	57400	D	61800	D	53100	D	25700	В	59500	D	63500	D	69400	E	64900	D	51500	С	63300	D	67600	E
I-71 CD	between Proposed Interchange and KY 53	n/a	n/a	n/a	n/a	n/a	n/a	9400		n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	21600		n/a	n/a	n/a	n/a
I-71 (11)	East of KY 53	38600	С	44600	С	44600	С	44600	С	44600	С	44600	С	58800	D	58800	D	58800	D	58800	D	58800	D
KY 146	West of I-71	8810	С	19200	F	19600	F	19400	F	19000	F	19300	F	25900	F	24400	F	24000	F	24900	F	25200	F
KY 146 (2)	between I-71 and KY 1817	12100	С	16200	D	15500	D	15600	D	15900	D	15900	D	24900	F	24000	F	24400	F	24000	F	24000	F
KY 146	between KY 1817 and KY 393	12400	С	12400	С	12100	С	13700	С	12500	С	12300	С	21200	F	20400	F	20600	F	20900	F	20500	F
KY 146	between KY 393 and KY 393	11600	С	12100	С	12100	С	14300	D	12600	С	12400	С	21600	F	21900	F	21400	F	22500	F	22100	F
KY 146 (12)	between KY 393 and KY 2854	8130	С	6400	С	6700	С	8300	С	7200	С	13200	С	11200	С	10600	С	10200	С	9900	С	18100	F
KY 146 (13)	between KY 2854 and KY 53	9570	С	7200	С	5400	С	7400	С	6700	С	10000	С	10700	С	12100	С	11500	С	11500	С	17200	F
KY 146	East of KY 53	5740	С	3100	В	4700	С	3100	В	3000	В	3300	В	6300	С	5400	С	6000	С	5700	С	6200	С
KY 393	South of KY 2856	5250	С	12100	С	12000	С	12300	С	11500	С	14800	D	16100	D	15600	D	15800	D	14100	D	16400	D
KY 393	between KY 2856 and I-71	6710	С	14300	D	13400	С	13600	С	13000	С	14200	D	19800	F	16900	E	17300	F	15100	D	16500	E
KY 393 (4)	between I-71 and KY 146	7720	С	9100	С	7600	С	8700	С	11300	С	6500	С	18800	F	15700	D	16200	D	21500	F	12400	С
KY 393	North of KY 146	4060	В	5200	С	5000	С	5000	С	5100	С	4800	С	12300	С	10200	С	10000	С	10800	С	10200	С
KY 53	South of KY 2856	6350	С	7600	С	6200	С	6300	С	6100	С	4800	С	23200	F	25000	F	25500	F	25800	F	17500	F
KY 53 (20)	between KY 2856 and connector	9030	С	12000	С	7000	С	6900	С	6900	С	6800	С	25200	F	19200	F	19400	F	18400	F	18100	F
KY 53 (10)	between and I-71	17300	С	14400	С	14400	С	14400	С	17800	С	21700	С	24600	С	18900	С	19200	С	18100	С	22100	С
KY 53 (9)	between I-71 and KY 146	15200	С	33000	E	28400	D	29500	D	30700	D	28900	D	42300	F	40600	F	40800	F	43800	F	41200	F
KY 53	North of KY 146	9200	С	18700	С	18700	С	18300	С	18500	С	18100	С	28000	D	27900	D	28100	D	27100	D	26500	D
Allen Ln (6)	between I-71 and Commerce	n/a	n/a	4500	С	7800	С	8200	С	5800	С	5700	С	21000	F	26000	F	26900	F	29200	F	28500	F
Allen Ln (15)	North of Commerce Pkwy	800	В	10900	С	5700	С	4600	С	3900	В	4200	В	12800	С	17300	F	16500	E	13700	С	14600	D
Commerce (14)	between KY 393 and Proposed Interchange	1200	В	1500	В	1300	В	1200	В	2400	В	2400	В	11500	С	11600	С	10900	С	17600	F	9000	С
Commerce (16)	between Proposed Interchange and KY 53	800	В	900	В	1900	В	1800	В	1900	В	1100	В	12800	С	10400	С	9600	С	15800	D	9300	С
Moody (17)	East of Proposed Overpass	1800	В	2100	В	8000	С	8300	С	3800	В	4400	С	4900	С	9700	С	9400	С	8400	С	9700	С
Moody (18)	West of Proposed Overpass	8900	C	10300	С	6800	С	7000	С	11200	С	12500	С	13900	D	13900	D	14300	D	11900	С	13300	С
Connector (7)	East of Proposed Overpass	n/a	n/a	2500	В	2700	В	2700	В	1100	В	1000	В	13700	C	10000	C	9500	С	14300	D	13100	С
Connector (19)	West of KY 53	n/a	n/a	1200	В	300	В	300	В	300	В	300	В	3300	В	1200	В	1300	В	2000	В	2200	В
KY 146 Ramp	from I-71 EB to KY 146	7200	E	8600	F	8400	F	9200	F	8700	F	8800	F	14100	F	11700	F	13200	F	13300	F	13500	F
KY 146 Ramp	from KY 146 to I-71 EB	6800	E	7800	F	8800	F	7900	F	8100	F	7900	F	11600	F	10000	F	12000	F	11800	F	11500	F
KY 146 Ramp	from K-71 WB to KY 146	10000	E	10500	F	11400	F	8800	F	10900	F	11200	F	12800	F	14500	F	14000	F	12800	F	13100	F
KY 146 Ramp	from KY 146 to I-71 WB	7900	E	9200	F	8500	F	7800	F	8900	F	8700	F	14400	F	15100	F	13200	F	13200	F	12900	F
KY 393 Ramp	from I-71 EB to KY 393	3300	E	4600	F	4500	F	5700	F	5200	F	6200	F	12200	F	12200	F	10500	F	13100	F	15600	F
KY 393 Ramp	from KY 393 to I-71 EB	2600	E	3100	F	3900	E	2600	E	1500	F	5100	F	5000	F	4700	F	3600	F	1800	F	6200	F
KY 393 Ramp	from K-71 WB to KY 393	2500	E	2900	F	1900	E	1200	E	1300	F	4900	F	4700	F	2100	F	1300	F	1500	F	5700	F
KY 393 Ramp	from KY 393 to I-71 WB	3100	E	4500	F	3900	F	4500	F	5200	F	6300	F	13700	F	11100	F	10500	F	13200	F	16100	F
New Ramp	from I-71 EB to connector					5400	F	4800	A							6700	F	5600	A				
New Ramp	from connector to I-71 EB					1100	D	700	A							3800	E	3500	A				
New Ramp	from K-71 WB to connector					1100	D	1100	A							4400	E	5600	A				
New Ramp	from connector to I-71 WB					5200	E	5500	A							6000	F	6200	A				
KV 52 Domo	from L 71 EB to KV 52	13400	D	16600		11000	D	4200	D	14800	E	14600	E	20100	F	12200	E	18500	E	17000	F	16900	F
KY 53 Ramp	from I-71 EB to KY 53		C	16600	F D	11000		4200	D		D	14600	E D			12200	E			17000		16800	
KY 53 Ramp	from KY 53 to I-71 EB	7200	C C	9500	D	8600	D	8700	D	9700	D	9500	D	21000	E	17100	F	17100	E	20900	E	20500	E
KY 53 Ramp	from K-71 WB to KY 53 from KY 53 to I-71 WB	7200	D	9700 14200	E	8700 10200	D	8400 5300	E	10000	E	8900 12500	E	22900 21900	F	18000 13200	E	15500 13600	E	23800 17600	F	21200 17100	E F
KY 53 Ramp	10111 KT 33 10 1-71 WB	12200	U	14200	E	10200	Ŭ	5300	E	10200	E	12500	E	21900	Г	13200	E	13000	E	17600	Г	17100	Г

Appendix C No Build

Traffic and LOS
Merge, Diverge, Weave 2008, 2015 and 2035



2008 EXISTING DETAILED ANALYSIS

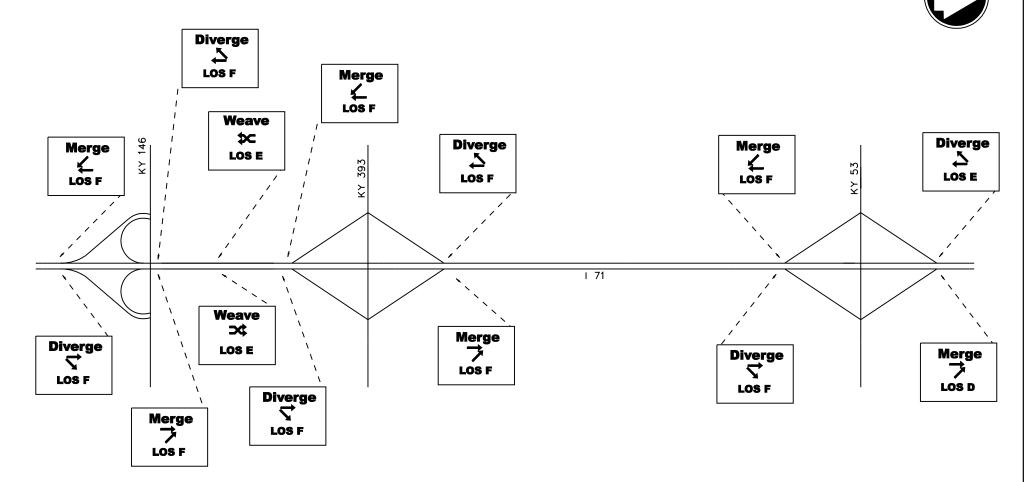


NOT TO SCALE

2015 NO-BUILD DETAILED ANALYSIS Diverge LOS F Merge Weave **\$C** Diverge 146 Diverge Merge Merge LOS D Δ Ł KY 393 Ł ¥ 53 LOS D LOS F LOS F LOS E ž 1 71 Weave ⊃¢ Merge Diverge ∽ LOS E Diverge Merge へ ァ LOS F LOS F LOS D LOS F Diverge へ Merge ブ LOS F LOS F

NOT TO SCALE

2035 NO-BUILD DETAILED ANALYSIS



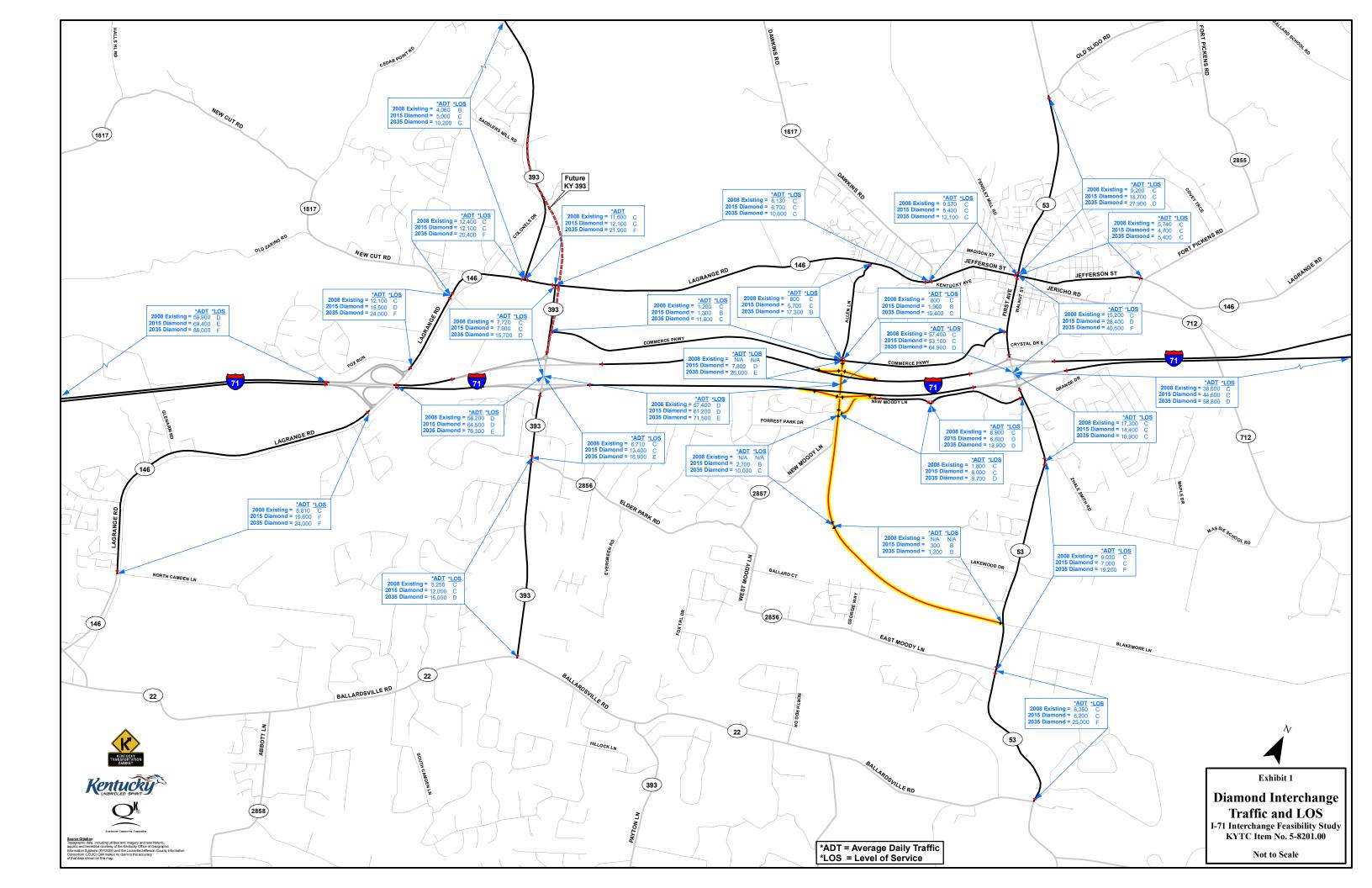
Appendix D Alternative 1 – Standard Diamond

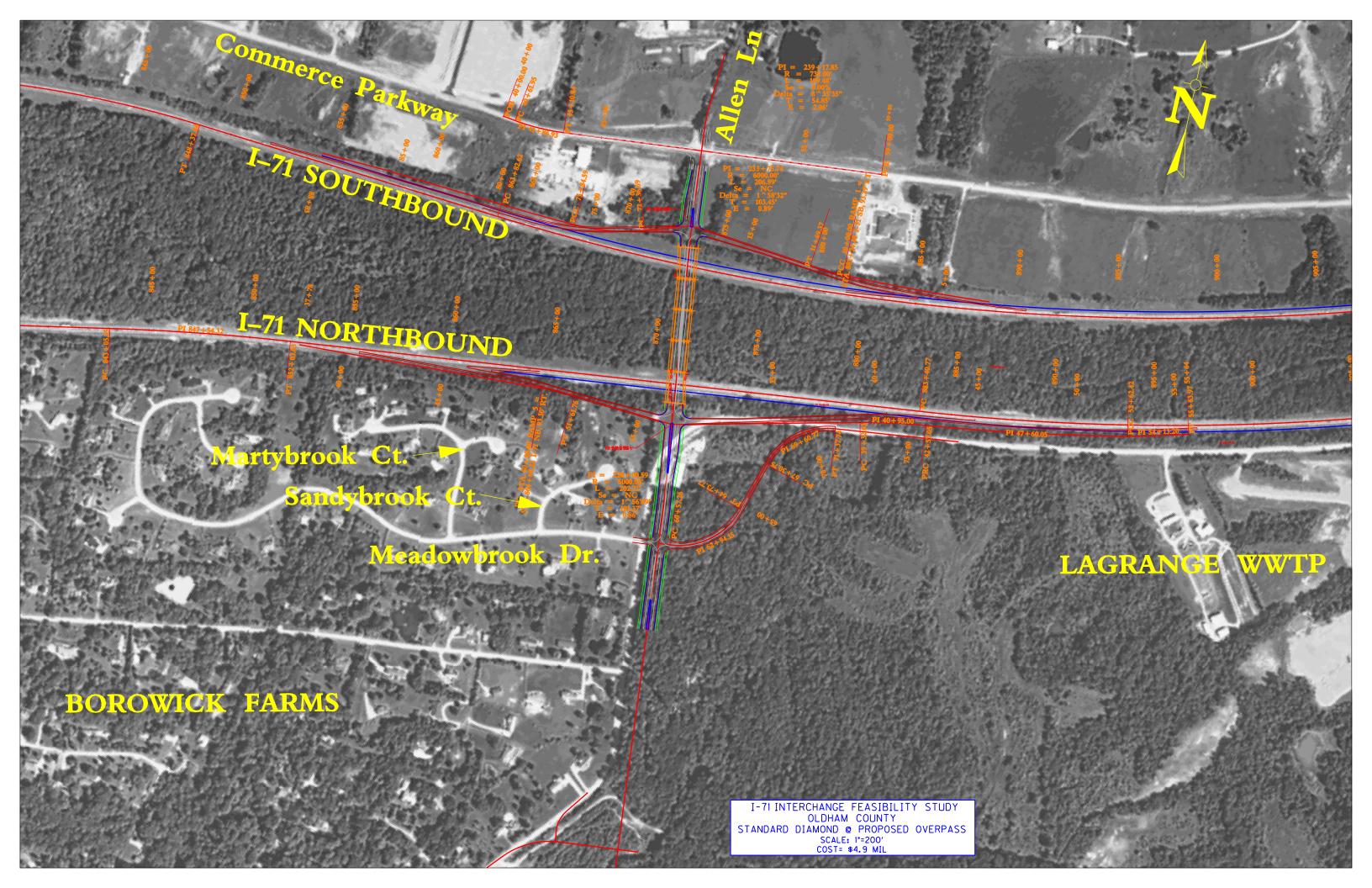
Traffic and LOS

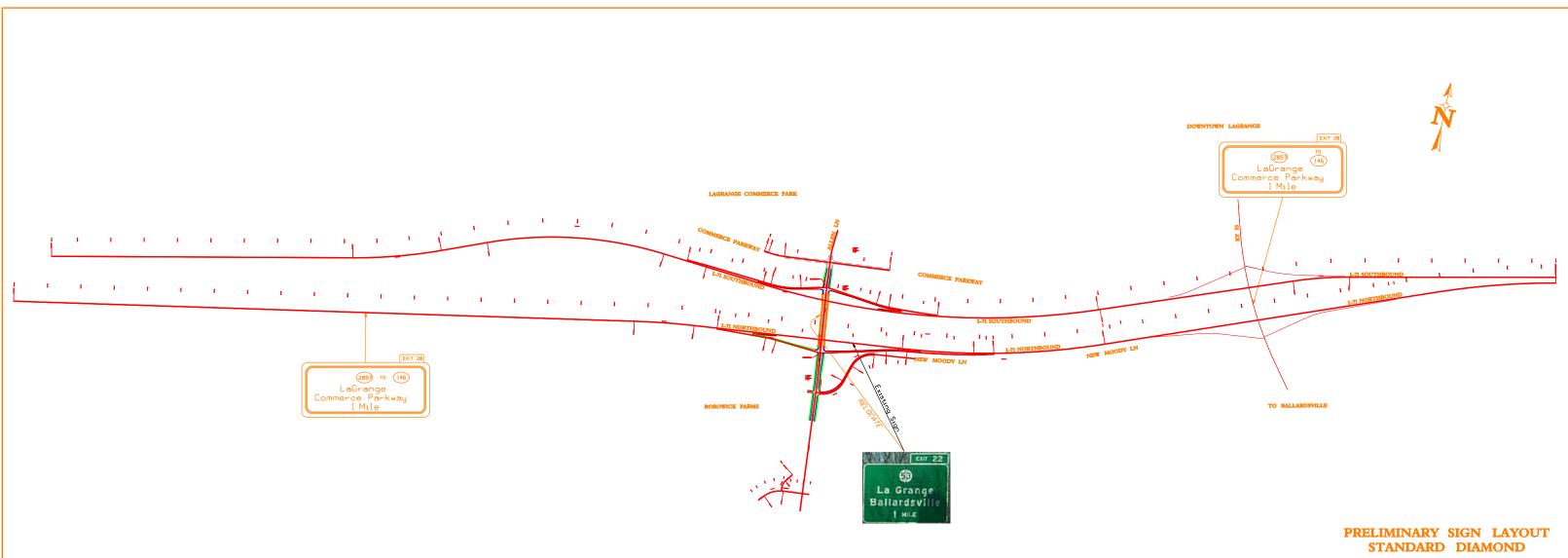
Preliminary Geometry

• Preliminary Sign Layout

• Merge, Diverge, Weave 2015 and 2035







2015 BUILD DIAMOND DETAILED ANALYSIS

